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To: Robert Mulka

Corrado Contracting, LLC

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From: Mary Flanagan, EIT

Fleis & VandenBrink Engineering

Date: May 8, 2024

Credit Union One Detroit, Michigan

Traffic Impact Assessment

1 Introduction

Re:

This memorandum presents the results of the Traffic Impact Assessment (TIA) for the proposed Credit Union One redevelopment in Detroit, Michigan. The project site is located at 1545 Junction Avenue, generally located in the south quadrant of Junction Avenue & Eldred Street intersection, as shown on the attached **Figure 1**. The project includes the construction of a new credit union on the property that includes the entire block between Eldred Street and Christiancy Street. The existing Credit Union One building and adjacent parking lot will be razed with the proposed project. Site access is proposed via two (2) driveways on Junction Avenue, which is under the jurisdiction of the City of Detroit.

Scope of this study was based on Fleis & VandenBrink's (F&V) knowledge of the study area, understanding of the development program, professional experience, accepted traffic engineering practice, and information published by Institute of Transportation Engineers (ITE). Sources of data for this study include information provided by ITE and the Michigan Department of Transportation (MDOT). Applicable background information is attached.

2 BACKGROUND DATA

2.1 EXISTING ROADWAY NETWORK

Vehicle transportation for the study area is provided via Junction Avenue. Information on the study roadway is summarized in **Table 1**. Additionally, the lane use and traffic control within the study roadway network is shown on the attached **Figure 2**.

Table 1: Roadway Information

Roadway Segment	Junction Avenue				
Number of Lanes	2 Lanes (1 lane each direction)				
Functional Classification	Major Collector				
Jurisdiction	City of Detroit				
Posted Speed Limit	30 mph				
Traffic Volumes (AADT)	6,800 (MDOT 2022)				

27725 Stansbury Boulevard, Suite 195 Farmington Hills, MI 48334

3 SITE TRIP GENERATION

The number of weekday peak hour (AM and PM) and daily vehicle trips that would be generated by the proposed development was forecast based on data published by ITE in the *Trip Generation Manual*, 11th *Edition*. The proposed development includes construction of a credit union with three (3) drive-thru lanes to replace the existing credit union on the property. The peak hour trip generation for both the AM and PM for the site is summarized in **Table 2**. The addition of the drive-through facility for this site is not anticipated to significantly increase the volume of trips generated by the development, only redistribute the existing trips from walk-in to drive-through.

	ITE			Average Daily Traffic	AM F	Peak Hou	r (vph)	PM P	eak Hou	ır (vph)	
La	and Use	Code	Amount	Units	(vpd)	In	Out	Total	ln	Out	Total
Existing	Walk - In Bank	911	3,025	SF	367	35	33	68	41	39	80
Proposed	Drive-in Bank	912	4,246	SF	426	33	30	63	45	44	89
Net New Tr	59	-2	-3	-5	4	5	9				

Table 2: Site Trip Generation Summary

4 SITE TRIP DISTRIBUTION

The vehicular trips that would be generated by the proposed development were assigned to the study roadway network based on the proposed site access plan and driveway configurations, existing peak hour traffic patterns in the adjacent roadway network, and methodologies published by ITE. This use is a neighborhood credit union and for purposes of this evaluation, it was assumed that trips generated by the development will be equality distributed both north and south of the site. The site trip distribution used in the analysis is summarized in **Table 3**.

Tak	tiibatio	11	
To/From	Via	AM	PM
North	Junction Avenue	50%	50%
South	Junction Avenue	50%	50%
	Total	100%	100%

Table 3: Site Trip Distribution

The vehicular traffic volumes shown in **Table 2** were distributed to the study network according to the distribution shown in **Table 3**. The projected future peak hour traffic volumes at the site driveways are shown on the attached **Figure 2**.

5 SITE CIRCULATION

The projected drive-through vehicle queuing was reviewed to determine if the proposed on-site drive-through storage is adequate to accommodate the projected operations. For purposes of this evaluation, it was assumed that 50% of the entering site generated trips would utilize the drive-through lanes.

The evaluation of the queue length included two criteria as summarized in Table 4:

- 1) A queuing analysis was performed to determine if the projected demand of the proposed development exceeds the service rate and calculate the projected queuing. The projected demand results in a queue of three (3) vehicles in the drive-through.
- 2) Additionally, a Poisson Distribution was performed to determine the probability of random arrivals; the results indicate a maximum potential of three (3) vehicles arriving at any given time.

The combined projected queue length is six (6) vehicles, in the three (3) lanes available, for a total of 150 feet. The proposed drive-through provides a total vehicle queueing storage of approximately 450 feet, between the 3 lanes, to accommodate 18 vehicles, at 25 feet per vehicle.

Therefore, the proposed development plan will adequately accommodate the projected vehicle queue lengths. In the event the vehicle demands exceed the drive-through capacity, the internal site circulation has adequate space to accommodate the additional vehicle storage on-site, without impacting the adjacent street operations along Junction Avenue.



CREDIT UNION ONE DRIVE-1 STACKING SPACE CALCU	
Number of Arrivals	23
Time per Vehicle (s)	180
Service Rate (veh/hr)	20
Peak Arrival (veh)	3
Random Arrival (veh)	3
Vehicle Length	25
TOTAL QUEUE (ft)	150

Table 4: Credit Union One Drive Through Vehicle Queuing Analysis

CONCLUSIONS

The conclusions of this TIA are as follows:

- 1. The proposed development includes construction of a credit union with three (3) drive-thru lanes to replace the existing credit union on the property. The addition of the drive-through facility for this site is not anticipated to significantly increase the volume of trips generated by the development, only redistribute the existing trips from walk-in to drive-through.
- 2. The proposed development plan will adequately accommodate the projected vehicle queue lengths. In the event the vehicle demands exceed the drive-through capacity, the internal site circulation has adequate space to accommodate the additional vehicle storage on-site, without impacting the adjacent street operations along Junction Avenue.

Any questions related to this memorandum, study, analysis, and results should be addressed to Fleis & VandenBrink.



I hereby certify that this engineering document was prepared by me or under my direct personal supervision and that I am a duly licensed Professional Engineer under the laws of the State of Michigan.

Digitally signed by
Julie M. Kroll
Date: 2024.05.08

Attachments: Figures 1-2

Proposed Site Plan Traffic Volume Data Poisson Queue Analysis







FIGURE 1 SITE LOCATION MAP

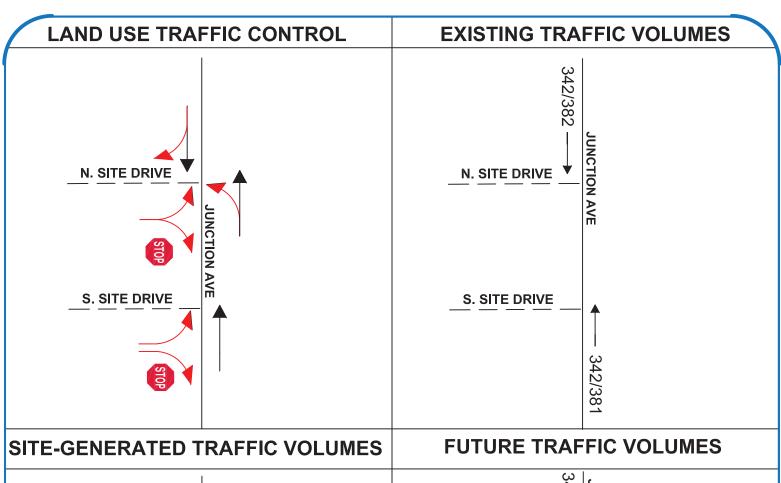
CREDIT UNION ONE TIA - DETROIT, MI

LEGEND



SITE LOCATION





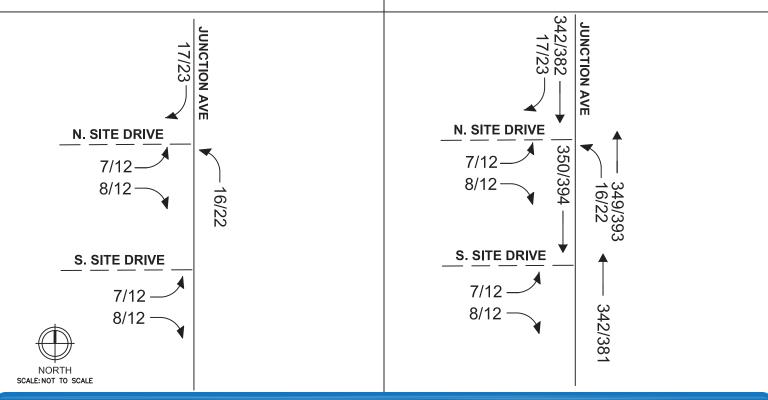
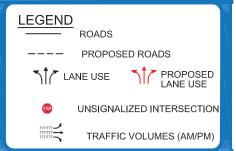
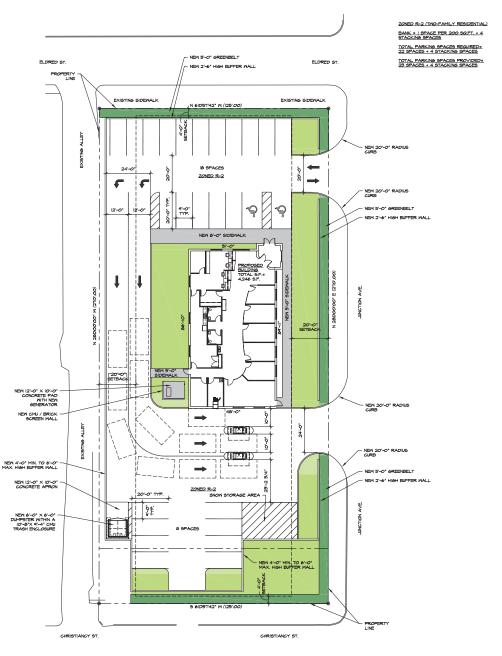




FIGURE 2 TRAFFIC VOLUMES

CREDIT UNION ONE TIA - DETROIT, MI









7300 DIXIE HWY. #600 CLARKSTON, MI 48346 PHONE 248.605.2030 FAX 248.605.2030 WEB DSARCHITECTS.COM

Seal

CREDIT UNION ONE DETROIT

Traving Title

1545 JUNCTION AVE. CITY OF DETROIT, MI 48

SITE PLAN NO. 6

t Number 22-40-162

recked DJS

1/16" = 1'0"

ONE OIT UNION ONE - SITE PLAN

DWNER REVIEW 3-2-23 DJS

Sheet Nun

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Transportation Data Management System

Disclaimer: The Michigan Department of Transportation (MDOT) works with individual agencies (cities/villages, counties, metropolitan planning organizations (MPOs), regional planning organizations (RPOs), and other areas of MDOT) to identify existing traffic count programs and/or traffic data. ... more

List View	All DIRs		Report Center
Record	of 1 Goto Record	go	
Location ID	82-1363	MPO ID	7970
Туре	SPOT	HPMS ID	
On NHS	No	On HPMS	No
LRS ID	6800165	LRS Loc Pt.	0.8355624
SF Group	Urban Non State	Route Type	
AF Group	NoFactor	Route	
GF Group	Urban Non State	Active	Yes
Class Dist Grp	NTL_5	Category	Primary
Seas Clss Grp			
WIM Group			
QC Group	Default		
Fnct'l Class	(5) Major Collector	Milepost	
Located On	JUNCTION		
Loc On Alias			
BETWEEN	Brandon St AND Plumer St		
More Detail 🕨			
STATION DAT	A		

Directions: 2-WAY NW SE



Src	ВС	PA	D %	K %	DHV-30	AADT	Year
Grown from 2022	188 (3%)	6,760 (97%)		11		6,948 ³	2023
	199 (3%)	6,599 (97%)		11	763	6,798	2022
Grown from 2020	278 (6%)	4,258 (94%)				4,536 ³	2021
Grown from 2019	279 (7%)	3,702 (93%)				3,981 ³	2020
Grown from 2018	188 (4%)	4,474 (96%)				4,662 ³	2019

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VOL			
	Date	Total	
9	Wed 3/2/2022	15	6,771
9	Tue 3/1/2022	15	6,825
9	Mon 11/8/2010	60	7,749
			10 10 10 10 10 10 10 10 10 10 10 10 10 1

VOLUME TREND 🗣)
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VOLUME TITLIND	
Year	Annual Growth
2023	2%
2022	50%
2021	14%
2020	-15%
2019	0%
2018	0%
2017	4%

CLASSIFICATION

95th Percentile Probability - Drive Through Queue Length (# of Vehicles)

Volume = 23 service rate = 60 λ = 0.383333

		1	2	3	4	5	6	7	8	9	
ĵ	λ^ x	No Veh in Cycle		X!	$P = (e^{-\lambda})(\lambda^{x})/X!$	Σ P	P* # Cycle containing Volume in 1	Σ Cycles in 6	Volume in Cycle (1*6)	Σ volume	Poisson Queue
	1.0000	0	0	1	68.16%	68.16%	41	41	0	0	NO
	0.3833	1	1	1	26.13%	94.29%	16	57	16	16	NO
	0.1469	2	2	2	5.01%	99.29%	3	11	6	22	NO
	0.0563	3	3	6	0.64%	99.93%	0	11	1	23	MET
	0.0216	4	4	24	0.06%	99.99%	0	11	0	23	MET
	0.0083	5	5	120	0.00%	100.00%	0	11	0	23	MET
	0.0032	6	6	720	0.00%	100.00%	0	11	0	23	MET
	0.0012	7	7	5040	0.00%	100.00%	0	11	0	23	MET
	0.0005	8	8	40320	0.00%	100.00%	0	11	0	23	MET
	0.0002	9	9	362880	0.00%	100.00%	0	11	0	23	MET
	0.0001	10	10	3628800	0.00%	100.00%	0	11	0	23	MET
	0.0000	11	11	39916800	0.00%	100.00%	0	11	0	23	MET