## Transportation Impact Study

for

## Project Panda



Prepared For:
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Suite 425
Rosemont, IL 60018
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August 2020
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(1) Design - Traffic: Signal Operations - Complex
(2) Design - Traffic: Capacity
(3) Design - Traffic: Safety Studies

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## TABLE OF CONTENTS

Section Page
I. EXECUTIVE SUMMARY ..... 4
Overview of Development ..... 4
Site Trip Generation, Distribution, and Capacity Analyses ..... 4
Conclusions ..... 5
II. INTRODUCTION / PROJECT SUMMARY ..... 6
Purpose of Report ..... 6
III. EXISTING STUDY AREA CONDITIONS ..... 6
Study Area Roadways ..... 6
Data Collection ..... 7
Existing Traffic Volume Growth. ..... 8
IV. DEVELOPMENT DESCRIPTION ..... 9
Proposed Site Access ..... 9
Site Trip Generation ..... 10
Site Trip Distribution ..... 10
V. FUTURE TRAFFIC VOLUMES ..... 11
No Build Traffic Volumes ..... 11
Build Traffic Volumes ..... 11
Roadway Improvements ..... 11
VI. OPERATIONAL ANALYSIS ..... 12
Capacity and Level of Service Analysis ..... 12
Traffic Signal Warrant Analyses ..... 13
VII. CONCLUSIONS ..... 13

## LIST OF FIGURES

Figure - Description
Figure 1 - Site Location Map
Figure 2 - Site Plan
Figure 3 - 2007 / 2019 Historic Peak Hour Traffic Volumes
Figure 4 - 2020 Historic Peak Hour Traffic Volumes
Figure 4A - 2020 Historic Regional Growth
Figure 5 - 2020 Existing Peak Hour Traffic Volumes
Figure 5A - Gateway Marketplace Trips
Figure 6 - 2020 Adjusted Peak Hour Traffic Volumes
Figure 6A - Traffic Volume Adjustment Factor
Figure 7 - 2022 No Build Peak Hour Traffic Volumes
Figure 7A - 2022 Regional Growth
Figure 8 - $\quad 2032$ No Build Peak Hour Traffic Volumes
Figure 8A - 2032 Regional Growth
Figure 9 - Phase I Site Trips
Figure 9A - Phase I Employee Site Trips
Figure 9B - Phase I Truck Site Trips
Figure 10 - Total Phase II Site Trips
Figure 10A - Phase II Employee Site Trips
Figure 10B - Phase II Truck Site Trips
Figure 11 - 2022 Phase I Build Peak Hour Traffic Volumes
Figure 12 - 2032 Master Plan Build Peak Hour Traffic Volumes

## LIST OF TABLES

Table - Description
Table 1A - Phase I Site Trip Generation
Table 1B - $\quad$ Phase II Site Trip Generation
Table 2A - AM Peak Hour Level of Service Comparison
Table 2B - PM Peak Hour Level of Service Comparison
Table 3A - AM Peak Hour Storage Length Comparison
Table 3B - PM Peak Hour Storage Length Comparison

## LIST OF APPENDICES

Appendix - Description
Appendix A - Turning Movement Counts
Appendix B - Field Inventory Sketches / Signal Plans
Appendix C - Approved Gateway Marketplace Trips Figure
Appendix D - MDOT Mode Split Correspondence
Appendix E - Level of Service Definitions
Appendix F - 2020 Existing Capacity Analysis
Appendix G - 2022 No Build Capacity Analysis
Appendix H - 2032 No Build Capacity Analysis

Appendix I - 2022 Phase I Build Capacity Analysis
Appendix J - 2032 Master Plan Build Capacity Analysis
Appendix K - Signal Warrant Analysis
Appendix L - Response to Comment Letter
Appendix M - 2032 Build ALT Volume Figures \& Capacity Analysis

## I. EXECUTIVE SUMMARY

## Overview of Development

Langan has prepared the following Transportation Impact Study in support of the proposed Project Panda development. As shown in Figure 1, the proposed project is located at the 1301 Eight Mile Road in the State Fair Grounds at the existing Gateway Marketplace in the City of Detroit, Michigan. The approximately 163.2 acre site is bound by Eight Mile Road (M-102) to the north, a CN rail line to the east, West State Fair Avenue to the south, and Woodward Avenue ( $\mathrm{M}-1$ ) to the west. The proposed warehouse (Warehouse C) will employ two (2) shifts of 993 employees and will include approximately 1,900 car parking spaces, 207 trailer parking spaces, and 64 loading docks. The majority of the day shift (@60\%) is anticipated to arrive between 6:30-7:30 AM prior to the typical AM commuter peak hour and depart between 5:30-6:30 PM after the typical PM commuter peak hour. Similarly, the majority of the night shift (@60\%) is anticipated to arrive between 5:30-6:30 PM after the typical PM commuter peak hour and depart between 4:30-5:30 AM prior to the typical AM commuter peak hour. In addition to relocating the existing transit station along Woodward Avenue to the State Fair Grounds property, there are also three (3) other proposed warehouses that are contemplated for a Phase II which total 665,000 SF (Warehouses B, D1, and D2).

The proposed site is to be constructed in two (2) phases of development. Phase I will include Warehouse C which is anticipated to be completed in 2022. Phase II will include the additional three (3) warehouses which are anticipated to be completed between 2026 and 2028. Access to the site is proposed along Eight Mile Road by one (1) full signalized access point, one (1) signalized access point along Woodward Avenue at the existing transit station, and two (2) full access unsignalized points along West State Fair Avenue with exiting stop control. Both access points along West State Fair Avenue will only service employee vehicles per the City of Detroit's request to restrict truck traffic along that road. The proposed site access locations are shown on Figure 2.

## Site Trip Generation, Distribution, and Capacity Analyses

Langan estimated the trip generation for the proposed warehouse which will utilize approximately 993 employees per shift and the combined 665,000 SF warehouses using trip generation data contained in the Trip Generation Manual, $10^{\text {th }}$ Edition, published by the Institute of Transportation Engineers (ITE). The end-user typically sees a $19 \%$ total modal split reduction for these types of facilities nationwide; however, for a conservative approach a $10 \%$ total modal split reduction was applied to the proposed site generated trips for transit, carpool, bicycles, and other uses. Based on data provided by the end-user of Warehouse C, trucks account for $3 \%$ of the peak hour site generated trips. Truck trips for warehouses B, D1, and D2 were calculated using the Trip Generation Manual Supplement, $10^{\text {th }}$ Edition, published by ITE.

The resulting trip generation calculations indicate the proposed Phase I development (Warehouse C) would generate 545 AM Peak Hour trips (392 In, 153 Out) and 589 PM Peak Hour trips ( 212 In, 377 Out). The resulting trip generation calculations indicate the proposed Phase II development (Warehouses B, D1, and D2) would generate an additional 140 AM Peak Hour trips (108 In, 32 Out) and 147 PM Peak Hour trips ( $40 \mathrm{In}, 107$ Out).

All proposed Phase I and Phase II trips were distributed to the study area network based on the location of the site and neighboring metropolitan areas, local employment statistics, on-site separation of truck and employee parking, and engineering judgement. The employee distributions estimated approximately $30 \%$ of site traffic will be coming to/from the northeast/east/southeast on Eight Mile Road, 20\% to/from the southwest on Woodward Avenue, 15\% to/from the northwest on Woodward Avenue, $15 \%$ to/from the east on West State Fair Avenue, $5 \%$ locally to/from the south on John R Street, and $15 \%$ locally to/from the west on Eight Mile Road. The truck distributions estimated approximately $55 \%$ of site traffic will be coming to/from the northeast/east/southeast on Eight Mile Road, $25 \%$ to/from the southwest on Woodward Avenue, 10\% to/from the northwest on Woodward Avenue, and $10 \%$ to/from the west on Eight Mile Road.

## Conclusions

The results of this study provide a broad overview of the transportation impacts that are associated with Project Panda. The proposed warehouses can be accommodated at this site by modifying the crossover east of Ralston Street to include two (2) eastbound left turning lanes and two (2) westbound left turning lanes. The northbound approach of Site Driveway A should include one (1) dedicated left turn lane and one (1) dedicated right turn lane. This system would also include updated traffic signal phasing to facilitate northbound exiting site traffic traveling across the eastbound Eight Mile Road segment to then turn left onto the westbound segment. The modification would include roadway widening / alterations and traffic signal upgrades at the intersection of Eight Mile Road \& Crossover / Site Driveway A. Additionally, signal modifications would be required at Woodward Avenue \& Site Driveway B to accommodate the new site connection. The westbound approach of Site Driveway B should include one (1) dedicated left turn lane and one (1) dedicated right turn lane. Minor signal timing optimization should also be pursued at Woodward Avenue \& West State Fair Avenue. Per MDOT's request, all three (3) of the aforementioned intersections should include upgraded or additional pedestrian facilities and infrastructure.

The capacity analyses indicate that all study area intersections are projected to operate at an overall LOS D or better during the 2022 Build and 2032 Build AM and PM Peak hour conditions.

In conclusion, the proposed warehousing development will have a minimal transportation impact on the surrounding study area roads \& intersections with the construction of the roadway improvement outlined above.

## II. INTRODUCTION / PROJECT SUMMARY

## Purpose of Report

The purpose of this document is to summarize the findings of the Transportation Impact Study conducted in support of the proposed Project Panda development. As shown in Figure 1, the proposed project is located in the State Fair Grounds in the City of Detroit, Wayne County, Michigan. The approximately 163.2 acre site is bound by Eight Mile Road (M-102) to the north, a CN rail line to the east, West State Fair Avenue to the south, and Woodward Avenue (M-1) to the west. There are currently two (2) existing access points to the site along Eight Mile Road, two (2) existing access points to the site along West State Fair Avenue, and one (1) existing access point to the site along Woodward Avenue. The proposed warehouse (Warehouse C) will employ two (2) shifts of 993 employees and will include approximately 1,900 car parking spaces, 207 trailer parking spaces, and 64 loading docks. The majority of the day shift (@60\%) is anticipated to arrive between 6:30-7:30 AM prior to the typical AM commuter peak hour and depart between 5:30-6:30 PM after the typical PM commuter peak hour. Similarly, the majority of the night shift (@60\%) is anticipated to arrive between 5:30-6:30 PM after the typical PM commuter peak hour and depart between 4:30-5:30 AM prior to the typical AM commuter peak hour. There are also three (3) other proposed warehouses which are proposed as part of the master plan development that total $665,000 \mathrm{SF}$ : Warehouse B 320,000 SF, Warehouse D1 - 195,000 SF, and Warehouse D2 - 150,000 SF.

There are a total of four (4) proposed access points to the site. One (1) access point (Site Driveway A) is proposed along Eight Mile Road which is proposed to become a fully signalized intersection which includes two (2) westbound left turn lanes at the crossover. This site access point will service both employee vehicles and trucks. There is one (1) signalized access point (Site Driveway B) proposed along Woodward Avenue (NB) at the existing transit station access. The transit station is proposed to be relocated onto the northwestern corner of the State Fair Grounds property adjacent to Site Driveway A and included with this development. The access point along Woodward Avenue is proposed to service employee vehicles, trucks, and transit vehicles and will need to be modified to accommodate the new site connection. There are two (2) unsignalized access points (Site Driveway C and Site Driveway D) proposed along West State Fair Avenue which are proposed to become full access driveways with exiting stop control. Both access points along West State Fair Avenue will only service employee vehicles per the City of Detroit's request. The proposed site plan is illustrated on Figure 2.

This report examines if there are any impacts from the proposed development on the surrounding intersections and roadways. Based on the results of the analyses, this report provides recommended improvements.

## III. EXISTING STUDY AREA CONDITIONS

## Study Area Roadways

Eight Mile Road (M-102) is a state road with a posted speed limit of 40 mph . This road has a general east-west orientation and is divided into two (2) segments by a median: one segment for eastbound vehicles and one segment for westbound vehicles. This road generally provides four (4) lanes of
travel in each direction with dedicated left turn lanes via crossovers. Land use along this road is predominately commercial.

Eight Mile Road (M-102) has multiple service roads to facilitate vehicles traveling to/from large arterial roads such as Woodward Avenue (M-102). These service roads have a general east-west orientation and generally provide two (2) lanes of travel in both directions with dedicated left turn lanes. Land use along these service roads are a mix of commercial and residential

Woodward Avenue ( $\mathrm{M}-1$ ) is a state road with a posted speed limit of 40 mph . This road has a general north-south orientation and is divided into two (2) segments by a median: one segment for northbound vehicles and one segment for southbound vehicles. This road generally provides five (5) lanes of travel in each direction with dedicated left turn lanes via crossovers. Land use along this road is predominately commercial.

Woodward Avenue (M-1) has two (2) service roads near the site to facilitate vehicles traveling to/from Eight Mile Road ( $\mathrm{M}-1$ ). These service roads have a general north-south orientation and generally provide two (2) to three (3) lanes of travel in both directions with dedicated left turn lanes. Land use along these service roads are predominately commercial.

State Fair Avenue is a city road with a posted speed limit of 30 mph near the site. This road has a general east-west orientation and provides one (1) lane of travel in each direction. Land use along this road is a mix of commercial and residential.

John R Street is a city road with a posted speed limit of 30 mph near the site. This road has a general north-south orientation and provides one (1) lane of travel in each direction. Land use along this road is predominately commercial.

## Data Collection

Based upon a review of the surrounding study area, the scope of the study identified to satisfy MDOT / City of Detroit requirements includes the peak hour turning movement counts at the following eight (8) intersections:

1. Eight Mile Road \& Site Driveway A (signalized)
2. E Woodward Service Road (NB) \& E Eight Mile Service Road (WB) (signalized)
3. E Woodward Service Road (SB) \& Eight Mile Service Road (EB) (signalized)
4. Woodward Avenue \& Site Driveway B (signalized)*
5. Woodward Avenue \& West State Fair Avenue (signalized)
6. West State Fair Avenue \& Site Driveway C / Ralston Street (unsignalized)*
7. West State Fair Avenue \& Site Driveway $D$ (unsignalized)*
8. West State Fair Avenue \& John R Street (signalized)**
(*Existing mainline thru volumes at these unsignalized intersections were determined from upstream or downstream signalized intersections. Additionally, Intersections 1 and 6 include estimated southbound and northbound volumes, respectively, based on engineering judgement as no historic count data included these turning movements.)
(**Turning movement counts at this intersection were conducted on July $16^{\mathrm{th}}, 2020$.)
Due to the current "Stay Home, Stay Safe" orders issued throughout Michigan and the nation abroad as a result of COVID-19, the collection of turning movement counts at the study intersections was not feasible initially. As such, two (2) different sets of historic count data was provided by MDOT. The first data set includes counts for Intersections 1, 2, and 3 from 2007 which were used in the approved Gateway Marketplace TIS. The second data set includes counts for Intersection 5 from 2019 which were used in an optimization project along Woodward Avenue.

The counts are representative of a typical weekday during the AM peak period (7:00 AM to 9:00 AM) and PM peak period (4:00 PM to 6:00 PM); however, the data from 2007 does not include typical turning movement count printouts but rather only summaries of the peak hour turning movements. All manual / video turning movement count summaries are included as Appendix A.

The AM and PM peak hours (four consecutive 15 -minute periods comprising the highest volume) from the intersection counts were used to determine the 2007 / 2019 Historic Peak Hour Traffic Volumes shown on Figure 3.

We conducted a desktop reconnaissance of the study area to obtain existing intersection geometry, turn lane lengths, lane widths, and posted speed limits. The inventory sketches and existing trafic signal plans are included in Appendix B.

## Existing Traffic Volume Growth

The Southeaster Michigan Council of Governments (SEMCOG) regional travel forecasting model was used to determine the regional growth rate in the area. Based on the model, and as previously identified in the approved Gateway Marketplace TIS, vehicular volumes are generally projected to decrease along the study roadways. As such, no growth was applied to the 2007 / 2019 historic count data so the volumes are representative of the standard "2019 peak hour traffic volumes".

For a conservative estimate, a growth rate of $1.0 \%$ per year was applied to all study roadways to grow these historic volumes from 2019 to 2020. The 2020 Historic Regional Growth is illustrated on Figure 4A. The 2020 Historic Regional Growth volumes were added to the 2007 / 2019 Historic Peak Hour Traffic Volumes (Figure 3) to obtain the 2020 Historic Peak Hour Traffic Volumes (Figure 4).

Since the 2007 count data was collected prior to the opening of the Gateway Marketplace, the trips outined in that study at Intersections 1, 2, and 3 (Figure 5A) were added to the 2007 / 2019 Historic Peak Hour Traffic Volumes (Figure 3). These trips were not included at other intersections as the Gateway Marketplace was open when the 2019 counts were conducted. The resulting 2020 Existing Peak Hour Traffic Volumes are illustrated on Figure 5. The original trip figure for the approved Gateway Marketplace study is included in Appendix C.

Per the City of Detroit's request, all existing turning movements at the John R Street \& West State Fair Street intersection which were collected in July 2020 were grown by $10 \%$ to account for potentially lower traffic volumes as a result of COVID-19. The additional volume growth at this intersection is illustrated on Figure 6A. This additional growth was added to the 2020 Existing Peak Hour Traffic Volumes to develop the 2020 Adjusted Peak Hour Traffic Volumes (Figure 6).

## IV. DEVELOPMENT DESCRIPTION

The proposed project is located at the 1301 Eight Mile Road in the State Fair Grounds at the existing Gateway Marketplace in the City of Detroit, Michigan. The approximately 163.2 acre site is bound by Eight Mile Road (M-102) to the north, a CN rail line to the east, West State Fair Avenue to the south, and Woodward Avenue to the west. The proposed Warehouse C will employ two (2) shifts of 993 employees and will include approximately 1,900 car parking spaces, 207 trailer parking spaces, and 64 loading docks. The majority of the day shift (@60\%) is anticipated to arrive between 6:30-7:30 AM prior to the typical AM commuter peak hour and depart between 5:30-6:30 PM after the typical PM commuter peak hour. Similarly, the majority of the night shift (@60\%) is anticipated to arrive between 5:30-6:30 PM after the typical PM commuter peak hour and depart between 4:30-5:30 AM prior to the typical AM commuter peak hour. There are also three (3) other proposed warehouses which are proposed as part of the master plan development that total $665,000 \mathrm{SF}$ : Warehouse B 320,000 SF, Warehouse D1 - 195,000 SF, and Warehouse D2 - 150,000 SF. In addition to the warehousing developments, the existing transit station located along Woodward Avenue is proposed to be relocated onto the northwestern corner of the State Fair Grounds property adjacent to Site Driveway A as part of this development.

The proposed site is to be constructed in two (2) phases of development. Phase I will include Warehouse C which is anticipated to be completed in 2022. Phase II will include the additional three (3) warehouses (Warehouses B, D1, and D2) which are anticipated to be completed between 2026 and 2028. Therefore, for the purposes of this study, Langan analyzed the following design scenarios:

- 2020 Existing Conditions
- 2022 Opening Day Conditions without Development (2022 No Build)
- 2022 Opening Day Conditions with Development (2022 Phase I Build)
- 2032 Opening Day Conditions without Development (2032 No Build)
- 2032 Opening Day Conditions with Development (2032 Master Plan Build)


## Proposed Site Access

There are a total of four (4) proposed access points to the site. One (1) access point (Site Driveway A) is proposed along Eight Mile Road which is proposed to become a fully signalized intersection which includes two (2) westbound left turn lanes at the crossover. This site access point will service both employee vehicles and trucks as well as relocated transit vehicles. There is one (1) signalized access point (Site Driveway B) proposed along Woodward Avenue (NB) at the existing transit station access. The transit station is proposed to be relocated onto the northwestern corner of the State Fair Grounds property adjacent to Site Driveway A and included with this development. The access point along Woodward Avenue is proposed to service employee vehicles, trucks, and transit vehicles and
will need to be modified to accommodate the new site connection. There are two (2) unsignalized access points (Site Driveway C and Site Driveway D) proposed along West State Fair Avenue which are proposed to become full access driveways with exiting stop control. Both access points along West State Fair Avenue will only service employee vehicles per the City of Detroit's request to restrict truck traffic along that road. The proposed site plan is illustrated in Figure 2.

## Site Trip Generation

Langan estimated the trip generation for the proposed warehouse with 993 employees per shift and the combined $665,000 \mathrm{SF}$ warehouses using trip generation data contained in the Trip Generation Manual, $10^{\text {th }}$ Edition, published by the Institute of Transportation Engineers (ITE). The end-user typically sees a $19 \%$ total modal split reduction for these types of facilities nationwide; however, for a conservative approach a $10 \%$ total modal split reduction was applied to the proposed site generated trips for transit, carpool, bicycles, and other uses. Correspondence with MDOT regarding the $10 \%$ modal split reduction is included in Appendix $\mathbf{D}$. Based on data provided by the end-user of Warehouse C, trucks account for $3 \%$ of the peak hour site generated trips. Truck trips for Warehouses B, D1, and D2 were calculated using the Trip Generation Manual Supplement, 10th Edition, published by ITE.

The resulting trip generation calculations indicate the proposed Phase I development (Warehouse C) would generate 545 AM Peak Hour trips (392 In, 153 Out) and 589 PM Peak Hour trips ( 212 In, 377 Out). The resulting trip generation calculations indicate the proposed Phase II development (Warehouses B, D1, and D2) would generate an additional 140 AM Peak Hour trips (108 In, 32 Out) and 147 PM Peak Hour trips ( $40 \mathrm{In}, 107$ Out). The Phase I site trip generation calculations are shown in Table 1A and the Phase II site trip generation for the additional warehouses are shown in Table 1B.

## Site Trip Distribution

All proposed Phase I and Phase II site generated trips were distributed to the study area network based on the location of the site and neighboring metropolitan areas, local employment statistics, on-site separation of truck and employee parking, and engineering judgement. The total proposed Phase I (Warehouse C) site generated trips are illustrated on Figure 8. The proposed Phase I employee site generated trips and distribution percentages are illustrated on Figure 8A. The proposed Phase I truck site generated trips and distribution percentages are illustrated on Figure 8B.

The total proposed Phase II (warehousing buildings B, D1, and D2) generated trips are illustrated on Figure 9. The proposed Phase II employee site generated trips and distribution percentages are illustrated on Figure 9A. The proposed Phase II truck site generated trips and distribution percentages are illustrated on Figure 9B.

The Phase I and Phase II employee distributions estimated approximately $30 \%$ of site traffic will be coming to/from the northeast/east/southeast on Eight Mile Road, $20 \%$ to/from the southwest on Woodward Avenue, 15\% to/from the northwest on Woodward Avenue, 15\% to/from the east on West State Fair Avenue, 5\% locally to/from the south on John R Street, and 15\% locally to/from the west
on Eight Mile Road. The Phase I and Phase II truck distributions estimated approximately 55\% of site traffic will be coming to/from the northeast/east/southeast on Eight Mile Road, $25 \%$ to/from the southwest on Woodward Avenue, 10\% to/from the northwest on Woodward Avenue, and 10\% to/from the west on Eight Mile Road.

## V. FUTURE TRAFFIC VOLUMES

## No Build Traffic Volumes

The Southeastern Michigan Council of Governments (SEMCOG) regional travel forecasting model was used to determine the regional growth rate in the area. Based on the model, and as previously identified in the approved Gateway Marketplace TIS, vehicular volumes are generally projected to decrease along the study roadways.

For a conservative estimate, a growth rate of $1.0 \%$ per year was applied to all study roadways to develop the 2022 regional growth volumes. Although first used to grow the 2007 / 2019 historic volumes to the 2020 historic volumes condition, the growth rate was also used to grow the 2020 historic volumes to the 2022 opening year condition and the 2032 design year condition. The 2022 Regional Growth (Figure 6A) was added to the 2020 Existing Peak Hour Traffic Volumes (Figure 5) to develop the 2022 No Build Peak Hour Traffic Volumes which are illustrated on Figure 6. The 2032 Regional Growth (Figure 7A) was added to the 2020 Existing Peak Hour Traffic Volumes (Figure 5) to develop the 2032 No Build Peak Hour Traffic Volumes which are illustrated on Figure 7.

## Build Traffic Volumes

We developed 2022 Phase I Build traffic volumes by adding the Total Phase I Site Trips (Figure 8) to the 2022 No Build Peak Hour Traffic Volumes (Figure 6). The 2022 Phase I Build Peak Hour Traffic Volumes are illustrated on Figure 10.

We developed 2032 Phase II Build traffic volumes by adding the Total Phase I (Warehouse C) Trips (Figure 8) and the Total Phase II (Warehouse Buildings B, D1, and D2) Trips (Figure 9) to the 2032 No Build Peak Hour Traffic Volumes (Figure 7). The 2032 Master Plan Build Peak Hour Traffic Volumes are illustrated on Figure 11.

## Roadway Improvements

To facilitate site traffic entering / exiting the development along Eight Mile Road, the eastbound-towestbound crossover located east of Ralston Street is proposed to facilitate both eastbound left and westbound left turning movements. The existing geometry of the crossover includes two (2) eastbound left turning lanes with a signalized intersection along the westbound segment of Eight Mile Road. The proposed geometry of the crossover includes two (2) eastbound left turning lanes and two (2) westbound left turning lanes. The westbound left turning lanes with allow site vehicles to turn left to travel south into Site Driveway A which will require the installation of a traffic signal along the eastbound segment of Eight Mile Road. The proposed geometry will also facilitate northbound
exiting site traffic to travel across the eastbound Eight Mile Road segment to then turn left onto the westbound segment. This modification would include roadway widening / alterations and traffic signal upgrades. Additionally, signal modifications would be required at Woodward Avenue \& Site Driveway $B$ to accommodate the new site connection as well as minor signal timing optimization at Woodward Avenue \& West State Fair Avenue.

## VI. OPERATIONAL ANALYSIS

## Capacity and Level of Service Analysis

Langan utilized the provided turning movement count data and existing roadway geometry and characteristics to perform capacity analyses based on Highway Capacity Manual (HCM) methodology for the study intersections. We used Synchro software to conduct the capacity analyses.

These analyses calculate the delay experienced by an average motorist and assigns the appropriate level of service (LOS). There are six levels of service that are defined for any intersection. They are given a letter designation from $A$ to $F$, with LOS A representing the best operating conditions and LOS F the worst. Typically, review agencies consider LOS D or better acceptable for urban conditions.

Table 1 \& Table 2 in Appendix E depicts the level of service criteria for signalized and unsignalized intersections.

Existing, No Build, Build, and Build with Improvements levels of service (LOS) were calculated for the AM and PM peak hours on a typical day for 2020 Existing, 2022 No Build, 2022 Opening Year, 2032 No Build, and 2032 Design Year. Roadway grades and lane widths were obtained through a desktop review which was incorporated into the calculations. Existing peak hour factors and heavy vehicle percentages were mostly left at their default values as the only count data printouts which included these values or 15 minute interval count data were for the Woodward Avenue \& West State Fair Avenue intersection.

The 2020 Existing, 2022 No Build, 2022 Phase I Build, 2032 No Build, and 2032 Master Plan Build levels of service are summarized in the Level of Service Comparison Tables 2A - 2B. The 2020 Existing, 2022 No Build, 2022 Phase I Build, 2032 No Build, and 2032 Master Plan Build Synchro printouts can be found in Appendix F - J, respectively. For capacity analyses reference purposes, traffic volumes and site trips at two (2) additional intersections (Int 9 - E Woodward Service Road (SB) \& Eight Mile Service Road (WB) and Int 10 - E Woodward Service Road (NB) \& Eight Mile Service Road (EB)) are included at the end of Appendix F.

As anticipated, there will be minimal increases in delay under the 2022 Build condition as a result of the increase in Phase I site traffic volumes. However, as shown in Tables 2A-2B, all study area intersections are projected to operate at an overall LOS D or better during both the AM and PM peak hours during the 2022 Build condition.

According to the capacity analyses, there will be minor increases in delay under the 2032 Build condition as a result of the increase in Phase II site traffic volumes. However, as shown in Tables 2A $\mathbf{- 2 B}$, all study area intersections are projected to operate at an overall LOS D or better during both the AM and PM peak hour during the 2032 Build condition.

A queue length comparison is provided for the AM and PM peak hours of all scenarios. As shown in Tables 3A - 3B, all 2022 Build and 2032 Build queues can be accommodated within the existing provided storage bay lengths.

## Traffic Signal Warrant Analyses

Based on the proposed improvements outlined in the Roadway Improvements section above, we conducted a peak hour signal warrant analysis, four-hour warrant analysis, and eight-hour warrant analysis for the intersection of Eight Mile Road (EB) \& Crossover / Site Driveway A. The eastbound segment of Eight Mile Road at Site Driveway A is currently unsignalized but the westbound segment at this location is currently signalized for protected eastbound left crossover and southbound right turning movements.

The peak hour signal warrants, four-hour signal warrants, and eight-hour signal warrants are based on MDOT guidelines and use the MDOT signal warrant spreadsheet. These warrants use historic ATR data provided by MDOT which were grown to 2022 and 2032 using the $1.0 \%$ linear growth rate. The reduced warrants were utilized as it can be assumed the $85{ }^{\text {th }}$ percentile speed is greater than the posted speed limit of 40 MPH along this multi-lane boulevard.

Based on the analysis, the Eight Mile Road (EB) \& Crossover / Site Driveway A intersection satisfies the peak hour warrant's criteria for both the AM and PM peak hours of the 2022 Phase I Build scenario and 2032 Master Plan Build scenario. This intersection also satisfies the four-hour warrant's criteria for the 2022 Build Phase I scenario and the 2032 Build Master Plan scenario as well as the eight-hour warrant's criteria for the 2032 Build Master Plan scenario.

Refer to Appendix K for the signal warrant calculations.

## VII. CONCLUSIONS

The results of this study provide a broad overview of the transportation impacts that are associated with Project Panda. The proposed warehouses can be accommodated at this site by modifying the crossover east of Ralston Street to include two (2) westbound left turning lanes and allowing northbound site vehicles exiting the proposed site to turn left at this intersection. This system would also include updated traffic signal phasing to facilitate northbound exiting site traffic traveling across the eastbound Eight Mile Road segment to then turn left onto the westbound segment. The modification would include roadway widening / alterations, vehicular and pedestrian signal upgrades, and additional pedestrian facilities / infrastructure upgrades at the intersection of Eight Mile Road \& Crossover / Site Driveway A. Additionally, signal modifications would be required at Woodward Avenue \& Site Driveway B to accommodate the new site connection as well as minor signal timing optimization at Woodward Avenue \& West State Fair Avenue. Pedestrian facilities and infrastructure
should be added / upgraded at the Woodward Avenue \& Site Driveway B intersection and Woodward Avenue \& West State Fair Avenue intersection.

The capacity analyses indicate that all study area intersections are projected to operate at an overall LOS D or better during the 2022 Build and 2032 Build AM and PM Peak hour conditions

In conclusion, the proposed warehousing development will have a minimal transportation impact on the surrounding study area roads \& intersections with the construction of the roadway improvement outlined above.

## FIGURES

Figure 1: Site Location Map
Figure 2: Site Plan
Figure 3: 2007 / 2019 Historic Peak Hour Traffic Volumes
Figure 4: 2020 Historic Peak Hour Traffic Volumes
Figure 4A: 2020 Historic Regional Growth
Figure 5: 2020 Existing Peak Hour Traffic Volumes
Figure 5A: Gateway Marketplace Trips
Figure 6: 2020 Adjusted Peak Hour Traffic Volumes
Figure 6A: Traffic Volume Adjustment Factor
Figure 7: 2022 No Build Peak Hour Traffic Volumes
Figure 7A: 2022 Regional Growth
Figure 8: 2032 No Build Peak Hour Traffic Volumes
Figure 8A: 2032 Regional Growth
Figure 9: Total Phase I Site Trips
Figure 9A: Phase I Employee Site Trips
Figure 9B: Phase I Truck Site Trips
Figure 10: Total Phase II Site Trips
Figure 10A: Phase II Employee Site Trips
Figure 10B: Phase II Truck Site Trips
Figure 11: 2022 Phase I Build Peak Hour Traffic Volumes
Figure 12: 2032 Master Plan Build Peak Hour Traffic Volumes






















## TABLES

Table 1A: Phase I Site Trip Generation
Table 1B: Phase II Site Trip Generation
Table 2A: AM Peak Hour Level of Service Comparison
Table 2B: PM Peak Hour Level of Service Comparison
Table 3A: AM Peak Hour Queue Length Comparison
Table 3B: PM Peak Hour Queue Length Comparison

TABLE IA
Project Panda
Site Trip Generation - Phase I

|  | ITE <br> Code | Size | Units | AM Peak Hour |  |  | PM Peak Hour |  |  | Weekday ADT |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Land Use |  |  |  | IN | OUT | TOTAL | IN | OUT | TOTAL |  |
| Proposed |  |  |  |  |  |  |  |  |  |  |
| Warehousing | 150 | 993 | Employees | 436 | 170 | 606 | $\underline{236}$ | 419 | 655 | 5,015 |
| Total Proposed Site Generated |  | 993 | Employees | 436 | 170 | 606 | 236 | 419 | 655 | 5,015 |
| Mode Split Reductions |  |  |  |  |  |  |  |  |  |  |
| Transit, Carpooling, Bicycles, etc. |  | 10\% |  | 44 | 17 | 61 | $\underline{24}$ | $\underline{42}$ | 66 | 502 |
| Total Mode Split Reductions |  | 10\% |  | 44 | 17 | 61 | 24 | 42 | 66 | 502 |
| Total Proposed External Site Generated |  |  |  | 392 | 153 | 545 | 212 | 377 | 589 | 4,5 I 3 |
| Vehicle Classification |  |  |  |  |  |  |  |  |  |  |
| Trucks |  | 3\% |  | 12 | 4 | 16 | 6 | 12 | 18 | 451 |
| Passenger Vehicles |  | 97\% |  | 380 | 149 | 529 | 206 | 365 | 571 | 4,062 |
| Total Vehicles |  | 100\% |  | 392 | 153 | 545 | 212 | 377 | 589 | 4,513 |

Notes:
Total national average mode split reduction for these facilities is $19 \%$ and expected mode split in Detroit is approximately $25 \%$ based on Census Bureau's American Community Survey from 2017. A 10\% mode split was approved by MDOT to provide a conservative approach.
Based on client provided data, trucks equate to approximately $3 \%$ of the peak hour site generated traffic.
Based on client provided data, trucks equate to approximately $10 \%$ of the development's ADT.

## LANGAN

TABLE IB
Project Panda
Site Trip Generation - Phase II

| Land Use | ITE <br> Code | Size | Units | AM Peak Hour |  |  | PM Peak Hour |  |  | Weekday ADT |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
|  |  |  |  | IN | OUT | TOTAL | IN | OUT | TOTAL |  |
| Proposed |  |  |  |  |  |  |  |  |  |  |
| Warehousing - Building B | 150 | 320,000 | SF | 49 | 15 | 64 | 18 | 48 | 66 | 551 |
| Warehousing - Building DI | 150 | 195,000 | SF | 38 | 11 | 49 | 14 | 37 | 51 | 354 |
| Warehousing - Building D2 | 150 | 150,000 | SF | $\underline{33}$ | 10 | 43 | 12 | 34 | 46 | $\underline{283}$ |
| Total Proposed Site Generated |  | 665,000 | SF | 120 | 36 | 156 | 44 | 119 | 163 | 1,188 |
| Mode Split Reductions |  |  |  |  |  |  |  |  |  |  |
| Transit, Carpooling, Bicycles, etc. |  | 10\% |  | 12 | 4 | 16 | 4 | 12 | 16 | 119 |
| Total Mode Split Reductions |  | 10\% |  | 12 | 4 | 16 | 4 | 12 | 16 | 119 |
| Total Proposed External Site Generated |  |  |  | 108 | 32 | 140 | 40 | 107 | 147 | 1,069 |
| Vehicle Classification |  |  |  |  |  |  |  |  |  |  |
| Trucks |  |  |  | 16 | 5 | 21 | 7 | 18 | 25 | 399 |
| Passenger Vehicles |  |  |  | $\underline{92}$ | $\underline{27}$ | 119 | $\underline{33}$ | 89 | 122 | 670 |
| Total Vehicles |  |  |  | 108 | 32 | 140 | 40 | 107 | 147 | 1,069 |

Notes:
Total national average mode split reduction for these facilities is $19 \%$ and expected mode split in Detroit is approximately $25 \%$ based on Census Bureau's American Community Survey from 2017. A 10\% mode split was approved by MDOT to provide a conservative approach.
Truck trips calculated using the ITE Trip Generation Manual Supplement, IOth Edition.

Table 2A
Level of Service

| AM / PM / SAT PEAK |  | AM PEAK HOUR (LOS / Delay) |  |  |  |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Direction | Approach / Movement | 2020 | 2022 Phase 1 |  |  | 2032 Master Plan |  |  |
|  |  | Existing | No Build | Build | Build with <br> Mitigation | No Build | Build | Build with <br> Mitigation |
| INTERSECTION |  | (1) Eight Mile Rd (WB) \& X/O / Site Driveway A |  |  |  |  |  |  |
| Eight Mile Rd (WB) |  |  |  |  |  |  |  |  |
| Westbound | Left Turn | B (10.6) | B (10.9) | C (33.0) |  |  | C (32.7) |  |
|  | Through |  |  | D (49.8) |  | B (12.6) | D (54.0) |  |
|  | Right Turn |  |  |  |  |  |  |  |
|  | Approach | B (10.6) | B (10.9) | D (49.1) |  | B (12.6) | D (53.0) |  |
| X/O/ Site Driveway A |  |  |  |  |  |  |  |  |
| Northbound | Left Turn | C (30.8) | C (30.8) | A (.5) |  | C (30.8) | A (.7) |  |
|  | Approach | C (30.8) | C (30.8) | A (.5) |  | C (30.8) | A (.7) |  |
| Southbound | Right Turn | D (41.1) | D (41.1) | B (17.2) |  | D (41.1) | B (19.2) |  |
|  | Approach | D (41.1) | D (41.1) | B (17.2) |  | D (41.1) | B (19.2) |  |
| OVERALL |  | B (11.5) | B (11.8) | D (46.7) |  | B (13.4) | D (50.5) |  |
| INTERSECTION |  |  |  |  |  |  |  |  |
| Eight Mile Rd (EB) |  | (1) Eight Mile Rd (EB) \& X/O / Site Driveway A |  |  |  |  |  |  |
| Eastbound | Left Turn | A (.0) | A (.0) | C (33.0) |  | A (.0) | D (37.9) |  |
|  | Through | A (.0) | A (.0) | B (15.9) |  | A (.0) | B (18.0) |  |
|  | Approach | A (.0) | A (.0) | B (16.9) |  | A (.0) | B (19.1) |  |
| X/O / Site Driveway A |  |  |  |  |  |  |  |  |
| Northbound | Through |  |  | C (33.2) |  |  | C (34.8) |  |
|  | Right Turn |  |  | C (32.1) |  |  | C (33.1) |  |
|  | Approach |  |  | C (32.5) |  |  | C (33.7) |  |
| Southbound | Left Turn |  |  | A (.8) |  |  | A (9) |  |
|  | Through |  |  |  |  |  |  |  |
|  | Approach |  |  | A (.8) |  |  | A (.9) |  |
| OVERALL |  | A (.0) | A (.0) | B (16.5) |  | A (.0) | B (18.5) |  |
| INTERSECTION |  | (2)Woodward Ave Srv Rd (NB) \& E 8 Mile Srv Rd (WB) |  |  |  |  |  |  |
| E 8 Mile Srv Rd (WB) |  |  |  |  |  |  |  |  |
| Westbound | Through | D (52.9) | D (52.7) | C (27.1) |  | D (46.0) | C (27.9) |  |
|  | Right Turn |  |  |  |  |  |  |  |
|  | Approach | D (52.9) | D (52.7) | C (27.1) |  | D (46.0) | C (27.9) |  |
| Woodward Ave Srv Rd (NB) |  |  |  |  |  |  |  |  |
| Northbound | Left Turn | A (1.1) | A (1.1) | A (1.4) |  | A (1.0) | A (1.2) |  |
|  | Through | A (2.8) | A (2.8) | A (2.8) |  | A (2.8) | A (2.8) |  |
|  | Approach | A (2.2) | A (2.2) | A (2.3) |  | A (2.2) | A (2.3) |  |
| OVERALL |  | C (24.3) | C (24.1) | B (13.1) |  | C (21.2) | B (13.4) |  |
| INTERSECTION |  | (3) Woodward Ave Srv Rd (SB) \& E 8 Mile Srv Rd (EB) |  |  |  |  |  |  |
| E 8 Mile Srv Rd (EB) |  |  |  |  |  |  |  |  |
| Eastbound | Through | C (28.1) | C (28.3) | C (28.3) |  | C (29.1) | C (29.1) |  |
|  | Right Turn |  |  |  |  |  |  |  |
|  | Approach | C (28.1) | C (28.3) | C (28.3) |  | C (29.1) | C (29.1) |  |
| Woodward Ave Srv Rd (SB) |  |  |  |  |  |  |  |  |
| Southbound | Left Turn | A (.5) | A (.5) | A (.5) |  | A (.8) | A (.7) |  |
|  | Through | A (1.5) | A (1.6) | A (1.3) |  | A (1.8) | A (1.5) |  |
|  | Approach | A (1.2) | A (1.3) | A (1.0) |  | A (1.5) | A (1.2) |  |
| OVERALL |  | B (15.4) | B (15.5) | B (14.7) |  | B (16.1) | B (15.2) |  |
| INTERSECTION |  | (4) Woodward Ave Srv Rd (NB) \& Site Driveway B |  |  |  |  |  |  |
| Site Driveway B |  |  |  |  |  |  |  |  |
| Westbound | Through | D (40.7) | D (40.7) | C (25.5) |  | D (40.8) | C (25.7) |  |
|  | Right Turn |  |  | C (25.2) |  |  | C (25.2) |  |
|  | Approach | D (40.7) | D (40.7) | C (25.3) |  | D (40.8) | C (25.4) |  |
| Woodward Ave Srv Rd (NB) |  |  |  |  |  |  |  |  |
| Northbound | Through | A (5.0) | A (5.0) | A (9.4) |  | A (5.0) | A (9.6) |  |
|  | Right Turn |  |  |  |  |  |  |  |
|  | Approach | A (5.0) | A (5.0) | A (9.4) |  | A (5.0) | A (9.6) |  |
| OVERALL |  | A (6.1) | A (6.1) | B (10.4) |  | A (6.1) | B (10.6) |  |
| INTERSECTION |  |  |  |  |  |  |  |  |
| Site Driveway B |  | (4) Woodward Ave Srv Rd(SB) \& Site Driveway B |  |  |  |  |  |  |
| Westbound | Left Turn | B (13.8) | B (13.8) | A (.2) |  | B (14.7) | A (.3) |  |
|  | Approach | B (13.8) | B (13.8) | A (.2) |  | B (14.7) | A (.3) |  |
| Woodward Ave Srv Rd (SB) |  |  |  |  |  |  |  |  |
| Southbound | Through | B (12.6) | B (12.9) | B (19.6) |  | B (14.8) | C (28.2) |  |
|  | Approach | B (12.6) | B (12.9) | B (19.6) |  | B (14.8) | C (28.2) |  |
| OVERALL |  | B (12.6) | B (12.9) | B (19.4) |  | B (14.8) | C (28.0) |  |

Table 2A
Level of Service


Table 2B
Level of Service

| AM / PM / SAT PEAK |  | PM PEAK HOUR (LOS / Delay) |  |  |  |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Direction | Approach / Movement | 2020 | 2022 Phase 1 |  |  | 2032 Master Plan |  |  |
|  |  | Existing | No Build | Build | Build with <br> Mitigation | No Build | Build | Build with Mitigation |
| INTERSECTION |  | (1) Eight Mile Rd (WB) \& X/O / Site Driveway A |  |  |  |  |  |  |
| Eight Mile Rd (WB) |  |  |  |  |  |  |  |  |
| Westbound | Left Turn | A (9.5) | A (9.7) | C (31.7) |  |  | C (32.0) |  |
|  | Through |  |  | C (27.1) |  | B (10.6) | C (29.7) |  |
|  | Right Turn |  |  |  |  |  |  |  |
|  | Approach | A (9.5) | A (9.7) | C (27.2) |  | B (10.6) | C (29.7) |  |
| X/O/ Site Driveway A |  |  |  |  |  |  |  |  |
| Northbound | Left Turn | C (31.0) | C (30.9) | A (.9) |  | C (31.2) | A (.8) |  |
|  | Approach | C (31.0) | C (30.9) | A (.9) |  | C (31.2) | A (.8) |  |
| Southbound | Right Turn | D (47.3) | D (47.3) | B (17.3) |  | D (47.3) | B (18.7) |  |
|  | Approach | D (47.3) | D (47.3) | B (17.3) |  | D (47.3) | B (18.7) |  |
| OVERALL |  | B (11.8) | B (12.0) | C (24.1) |  | B (12.7) | C (26.4) |  |
| INTERSECTION |  | (1) Eight Mile Rd (EB) \& X/O / Site Driveway A |  |  |  |  |  |  |
| Eight Mile Rd (EB) |  | (1) Eigh Me Rd(Eb) \& X/O / Ste Diveway A |  |  |  |  |  |  |
| Eastbound | Left Turn | A (.0) | A (.0) | C (31.7) |  | A (.0) | D (41.7) |  |
|  | Through | A (.0) | A (.0) | B (19.4) |  | A (.0) | C (26.9) |  |
|  | Approach | A (.0) | A (.0) | C (20.7) |  | A (.0) | C (28.3) |  |
| X/O / Site Driveway A |  |  |  |  |  |  |  |  |
| Northbound | Through |  |  | D (36.1) |  |  | D (37.9) |  |
|  | Right Turn |  |  | C (32.7) |  |  | C (33.1) |  |
|  | Approach |  |  | C (33.9) |  |  | C (34.7) |  |
| Southbound | Left Turn |  |  | A (.4) |  |  | A (.5) |  |
|  | Through |  |  |  |  |  |  |  |
|  | Approach |  |  | A (.4) |  |  | A (.5) |  |
| OVERALL |  | A (.0) | A (.0) | C (21.0) |  | A (.0) | C (28.1) |  |
| INTERSECTION |  | (2)Woodward Ave Srv Rd (NB) \& E 8 Mile Srv Rd (WB) |  |  |  |  |  |  |
| E 8 Mile Srv Rd (WB) |  |  |  |  |  |  |  |  |
| Westbound | Through | D (43.6) | D (42.9) | C (28.5) |  | D (40.5) | C (29.5) |  |
|  | Right Turn |  |  |  |  |  |  |  |
|  | Approach | D (43.6) | D (42.9) | C (28.5) |  | D (40.5) | C (29.5) |  |
| Woodward Ave Srv Rd (NB) |  |  |  |  |  |  |  |  |
| Northbound | Left Turn | A (.3) | A (.4) | A (.3) |  | A (.4) | A (.4) |  |
|  | Through | A (2.0) | A (2.0) | A (1.9) |  | A (2.1) | A (2.0) |  |
|  | Approach | A (1.4) | A (1.5) | A (1.4) |  | A (1.6) | A (1.5) |  |
| OVERALL |  | B (17.0) | B (16.8) | B (11.5) |  | B (16.0) | B (12.0) |  |
| INTERSECTION |  | (3) Woodward Ave Srv Rd (SB) \& E 8 Mile Srv Rd (EB) |  |  |  |  |  |  |
| E 8 Mile Srv Rd (EB) |  |  |  |  |  |  |  |  |
| Eastbound | Through | C (28.8) | C (28.9) | C (28.9) |  | C (29.7) | C (29.7) |  |
|  | Right Turn |  |  |  |  |  |  |  |
|  | Approach | C (28.8) | C (28.9) | C (28.9) |  | C (29.7) | C (29.7) |  |
| Woodward Ave Srv Rd (SB) |  |  |  |  |  |  |  |  |
| Southbound | Left Turn | A (.8) | A (.9) | A (.8) |  | A (1.2) | A (1.1) |  |
|  | Through | A (1.4) | A (1.5) | A (1.4) |  | A (1.6) | A (1.5) |  |
|  | Approach | A (1.2) | A (1.3) | A (1.2) |  | A (1.5) | A (1.4) |  |
| OVERALL |  | B (14.6) | B (14.7) | B (14.4) |  | B (15.4) | B (15.0) |  |
| INTERSECTION |  | (4) Woodward Ave Srv Rd (NB) \& Site Driveway B |  |  |  |  |  |  |
| Site Driveway B |  |  |  |  |  |  |  |  |
| Westbound | Through | D (41.2) | D (41.2) | C (31.0) |  | D (41.4) | C (31.4) |  |
|  | Right Turn |  |  | C (31.2) |  |  | C (31.7) |  |
|  | Approach | D (41.2) | D (41.2) | C (31.1) |  | D (41.4) | C (31.5) |  |
| Woodward Ave Srv Rd (NB) |  |  |  |  |  |  |  |  |
| Northbound | Through | A (5.4) | A (5.5) | B (16.0) |  | A (5.7) | B (19.0) |  |
|  | Right Turn |  |  |  |  |  |  |  |
|  | Approach | A (5.4) | A (5.5) | B (16.0) |  | A (5.7) | B (19.0) |  |
| OVERALL |  | A (5.8) | A (5.8) | B (16.5) |  | A (6.1) | B (19.5) |  |
| INTERSECTION |  | (4) Woodward Ave Srv Rd (SB) \& Site Driveway B |  |  |  |  |  |  |
| Site Driveway B |  |  |  |  |  |  |  |  |
| Westbound | Left Turn | D (37.7) | D (37.7) | A (.4) |  | D (37.7) | A (.6) |  |
|  | Approach | D (37.7) | D (37.7) | A (.4) |  | D (37.7) | A (.6) |  |
| Woodward Ave Srv Rd (SB) |  |  |  |  |  |  |  |  |
| Southbound | Through | A (7.3) | A (7.4) | A (9.2) |  | A (7.6) | A (9.5) |  |
|  | Approach | A (7.3) | A (7.4) | A (9.2) |  | A (7.6) | A (9.5) |  |
| OVERALL |  | A (7.6) | A (7.3) | A (8.9) |  | A (7.5) | A (9.1) |  |

Table 2B
Level of Service


Table 3A

## Queue Table



Table 3A

## Queue Table



Notes:
$\left(X^{\prime}\right)$ represents the proposed queue length under the Build conditions.

Table 3B

## Queue Table



Table 3B

## Queue Table



Notes:
$\left(X^{\prime}\right)$ represents the proposed queue length under the Build conditions.

